







November, 2005

SR-68, 2600 South to I-15 in Davis County SR-68 **Project No. SP-STP-0068(21)68E** 2600 South



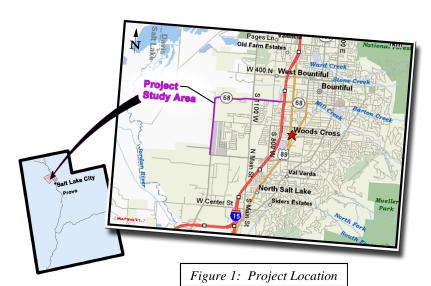
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#### Introduction

This technical memorandum documents the existing infrastructure conditions, evaluation of geometric elements, and accident information for

the SR-68/500 South corridor. from the intersection of 2600 South and Redwood Road to the southbound ramps west of I-15, in West Bountiful and Woods Cross. Utah. Refer to the project location in Figure 1. This section of SR-68 is considered an



urban principal arterial and serves as a link between I-15, future commuter rail and the proposed Legacy Parkway. The Utah Department of Transportation (UDOT) right-of-way width is 100 feet from 2600 South on Redwood Road to 800 West on 500 South. Right-of-way along the corridor, from the intersection of 2600 South and Redwood Road to approximately the intersection of 800 West and 500 South, is 100 feet. From 800 West to I-15, the right-of-way width narrows to a total width of 66 feet. The following elements are detailed in this report:

Section 1.0 Geometrics Section 2.0 Pavement Section 3.0 Structures Section 4.0 Drainage Section 5.0 Irrigation/Canals Section 6.0 Utilities Section 7.0 Crash Data

Figures A1 to A12 in the Appendix provide a detailed layout of existing information including: curve speed, shoulder widths, clear zone issues, crash data and posted speed limits. Figures B1 to B12 show approximate locations of utilities along the corridor. Figures C1 to C4 provide design criteria and evaluations of the current roadway geometrics. Figures D1 to D12 show existing drainage features.

#### 1.0 Geometrics

Roadway design standards from the Utah Department of Transportation (UDOT) and the American Association of State Highway

> **Transportation Officials** (AASHTO) were used to evaluate the existing roadway conditions. Roadway deficiencies are determined based on an evaluation of the existing conditions of the corridor compared to the design standards. The existing geometrics of SR-68 were determined using the 500 South Corridor Needs Assessment, field reviews, survey data, aerial mapping and interviews with stakeholders. All of the

gathered information was compared to the design criteria.

#### 1.1 Critical Elements

Design standards used are based upon UDOT and Federal Highway Administration (FHWA) 12 critical design elements. These 12 elements include:

- Design Speed
- · Lane Width
- Shoulder Width
- Horizontal Alignment
- Vertical Alignment
- Grades
- Stopping Sight Distance
- Cross Slopes
- Superelevation
- Structural Capacity
- Vertical Clearance
- Bridge Width

To obtain the roadway deficiencies, the existing conditions of the corridor were compared to these design elements. The deficiencies found are outlined below. Detailed layouts of existing deficiencies are shown in Figures A1-A12.

#### 1.2 Design Speed

Design standards are based upon the design speed of the corridor. For this urban arterial, the recommended design speed can vary from 30-60 mph depending on the setting of the roadway. The setting or context of SR-68 has three sections, which are generally delineated by the posted speed limit of each section. These sections and their posted speed limits are:

- 2600 South to RP 67 (curve at Redwood Road and 500 South) 55 mph
- RP 67 to 1100 West 45 mph
- 1100 West to I-15 35 mph

The design speed is generally 5 mph greater than the posted speed limit. Design criteria is based on the design speed. The design criteria table developed for this cooridor is outlined in Figure C1.



45 mph speed zone & abrupt pavement taper after 1100 West Inventory and analysis data tables are included in Figures C2 – C4. Deficient items (those that do not meet the design criteria) are in bold lettering.

#### 1.3 Analysis

Deficiencies in

roadway geometrics often contribute to poor traffic operations and crashes. Geometric elements that are closely associated to traffic operations and crashes are vertical alignment, horizontal alignment and cross sectional features.

The current roadway geometrics have not changed substantially since the road was constructed in 1953. Since its construction, minor roadway projects have brought many inconsistent element changes to the roadway. Minor maintenance projects are the only projects on record since the construction of the roadway.

#### 1.4 Horizontal Alignment

The radius (or sharpness) of the curve, appropriate

superelevation (amount of bank on the roadway), and proper sight distance (horizontal sightline offset) on horizontal curves are essential for safe maneuvering along horizontal alignments.

On this corridor, there are 14 horizontal curves. All but one of these curves are minor horizontal

curves and are well within the design criteria for the corridor. There is one main existing horizontal curve (RP 67 – curve from Redwood Road to 500 South) that meets a design speed of 15 mph (radius of 500 feet, 3% max superelevation). Posted speed prior to this curve is 45 mph from each direction. As a result, this curve



Looking West before the horizontal curve at 500 South & Redwood Road

does not meet the criteria for the posted speed limit. An advisory speed limit sign of 30 mph is provided.

The horizontal sightline offset distances provided for curves on the corridor are sufficient.

Intersection geometry at several intersections is deficient. Acceleration and deceleration lanes are inadequate. The following is a summary of the intersections with deficiencies:

- 2600 South & Redwood Road
- 2425 South & Redwood Road
- 1500 South & Redwood Road
- 1180 South & Redwood Road
- 1100 West & 500 South
- I-15 Southbound On-Ramp & 500 South

Table 1 contains a detailed analysis of deficient acceleration and deceleration lanes found on the corridor. Values in bold indicate a deficient length

		Acceleration				Deceleration			
Intersection	Design Speed	Speed Change Length		Taper Length		Speed Change Length		Taper Length	
		Provided	Req'd	Provided	Req'd	Provided	Req'd	Provided	Req'd
2600 South & Redwood Road	60 mph	200 ft	1200 ft	285 ft	300 ft	360 ft	500 ft	o ft	180 ft
2425 South & Redwood Road	60 mph	200 ft	1200 ft	N/A	N/A	300 ft	500 ft	180 ft	180 ft
1500 South & Redwood Road	60 mph	280 ft	1200 ft	180 ft	300 ft	380 ft	500 ft	160 ft	180 ft
1180 South & Redwood Road	60 mph	40 ft	1200 ft	N/A	N/A	280 ft	500 ft	130 ft	180 ft
1100 West & 500 South (West of Intersection)	50 mph	250 ft	720 ft	o ft	300 ft	220 ft	405 ft	o ft	180 ft
1100 West & 500 South (East of Intersection)	40 mph	550 ft	360 ft	o ft	300 ft				
I-15 SB On- Ramp	25 mph	1		1	1	None	295 ft	N/A	N/A

Table 1: Existing & Required Acceleration & Deceleration Lanes--Bold numbers denote insufficient lane lengths

for the lane.

#### 1.5 Vertical Alignment

Crashes on roadways are often attributed to inadequate sight distance. Sight distance is the length of the roadway ahead that is continuously visible to the driver and is an important concept of roadway design related to stopping, steering and overtaking. Stopping sight distance is the sufficient distance necessary to enable a vehicle traveling at the design speed to stop before reaching a stationary object in its path.

There are 10 vertical curves in the project area. All of these vertical curves meet current design standards. Existing grades on the corridor vary from 0.03% to 2.91%. The maximum recommended grade for this roadway to meet the design standards is 5% to 7%. The desired minimum grade for this corridor is 0.5%, however the absolute minimum for drainage is 0.3%. Flat grades can cause poor drainage of the roadway which in turn compromises safety and traffic operations. There are three grades along the study corridor that do not meet the minimum grade of 0.5%. Just south of 1500 South is a 0.03% grade,

the middle of the curve at RP 67 has a -0.18% grade and just east of 1500 West is a 0.11% grade. For more information on the vertical alignment, refer to Figure C3.

#### 1.6 Cross Section Elements

Cross section elements that contribute to the safety of the roadway include cross slope of the roadway, clear zone, sideslopes, lane widths and shoulder widths. The existing cross section of SR-68 has a variety of deficiencies, as shown in Figures A1 to A12 and Table 2. Table 2 outlines the current cross section features of the SR-68 study area.

The following is a summary of the existing cross section elements:

#### Traffic Lanes

The traffic lane width of a roadway greatly influences the safety and comfort of driving. Lane width also affects the roadway's level of service. Narrow lanes force drivers to operate their vehicles closer laterally to each other than they would normally desire. Lane widths of 12 feet are recommended on most arterials. The through lanes on this corridor are generally sufficient in

in

width (12 feet). However, there are areas along 500 South near I-15, where the lane widths are 11 feet.

#### Shoulders

A shoulder is the portion of the roadway contiguous with the traveled way that accommodates stopped vehicles, emergency use, and lateral support of roadway pavement. Shoulder width should be consistent throughout a corridor. In some cases, the shoulder can accommodate bicyclists. Shoulders can also serve as speed-change lanes for vehicles turning into driveways. Roadways with high truck traffic should have wider shoulders, however, wider shoulders can encourage unauthorized use of the shoulder as a travel lane or on-street parking.

This corridor has a recommended shoulder width of 8 feet. The percentage of trucks on the corridor is 7-8%. The existing shoulders range from 0-30 feet wide. Most of the corridor (from 2600 South to 800 West) has a deficient shoulder of 1 foot. Areas where the shoulders are 27-30 feet wide are sections of relatively new development where a right-of-way setback was required. The shoulder width in these areas, near 1100 West and 700 West, encourages on-street parking. See Figures A1 to A12 for locations of inconsistent shoulder widths.

#### Clear Zone

Clear zone is the area adjacent to the travel way that is clear of obstructions that could impede the safe recovery of a vehicle that has left the traveled way. Obstructions that can not be removed from the clear zone are shielded with safety devices known as end treatments, which protect the motorist from more serious injury/damage that may occur if the vehicle were to hit the obstruction.

Clear roadside design is recommended for urban arterials whenever practical. Clear zone area is dependant on the context of the roadway cross section and the design speed of the corridor. The context of this corridor is delineated by sections with and without curb and gutter. In sections without curb and gutter, the width of the clear zone area is dependant on the design speed. From 2600 South to RP 67 the minimum clear zone is 30 feet. From RP 67 to 1100 West the minimum clear zone is 16 feet.

The section of the corridor from 1100 West to I-15 generally has curb and gutter. In this section, the clear roadsides described above are often impractical. In such areas, a minimum offset of 1.5 feet between curb face and object is recommended.

There are many clear zone hazards throughout the corridor. Obstructions include power poles on the east side of Redwood Road and the south side of

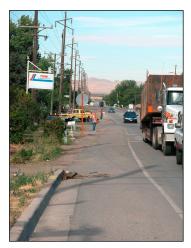
Corridor Sections	Design Speed	Traffic Lane Width	Shoulder Width	Sideslope	Cross Slope	Clear Zone
	mph	Feet	Feet	Feet : Feet	%	Feet
Recommended Values	1	12	8	4:1 or flatter	2	SECTION 1: 30 SECTION 2: 18 SECTION 3: 3
SECTION 1: 2600 South Redwood Road to RP 67	60	12	1	Varies (See Figures A1-A12)	0.2 - 3.6 (See Figure A-5)	Varies
SECTION 2: RP 67 to 1100 West and 500 South	50	12	Varies 1 – 30	Varies (See Figures A1-A12)	2	Obstructions (See Figures A1-A12)
SECTION 3: 1100 West and 500 South to I-15	40	11—12	Varies 1 – 30	6:1 or flat- ter	1.5-2.0	Obstructions (See Figures A1-A12)

Table 2: Existing Cross Section Elements

500 South, drainage ditches, mailboxes and signs, inadequate protection of railroad apparatus at the Denver & Rio Grande Western Railroad (D&RGW) crossing, and roadside ditches. See Figures A1 to A12 for specific areas of clear zone deficiencies.

#### <u>Sideslopes</u>

Sideslope is the area from the paved shoulder to the outer edge of the clear zones. Sideslope is important to ensure



Inconsistent shoulder widths & curb & gutter near 800 West

roadway stability and to provide a reasonable opportunity for recovery for an out-of-control vehicle. Critical to this area is the foreslope (area from the edge of roadway surface and the toe of the slope) and the backslope (area from the toe of the slope and back slope that forms a ditch). Generally the minimum side slope should be 4:1. If a roadside ditch is formed, then the ditch should generally be a 6:1 with a 3:1 foreslope/backslope combination. Figures A1 to A12 identify locations with green hatch marks where sideslopes within the clear zone are steeper than 4:1. Existing deficiencies of the roadside sideslopes vary throughout the corridor, however, most of the deficiencies occur in the section from 2600 South to 1100 West due to drainage ditches with steep sideslopes.

#### Cross Slope

Sufficient cross slope for adequate pavement drainage is important on arterials. Typical problems related to splashing and hydroplaning are compounded by heavy traffic volumes and curbed sections, especially for high speeds. The existing cross slope for the SR-68 corridor ranges from 1.5% - 3.0%. This is an acceptable range for cross slopes.

#### **Sidewalks**

Deficiencies in pedestrian facilities include sidewalk gaps, lack of handicap ramps and inadequate layout and slope requirements per the ADA Accessibility Guidelines for Buildings and Facilities Standards. Although cycling groups use the corridor regularly, the entire corridor does not have any designated bike lanes or equestrian trails.

From 1100 West to I-15, the edge of roadway is defined by curb and gutter. Pedestrian use along the corridor is desirable by the community; however, there are few intermittent sidewalks found mostly between 1100 West and I-15. **Both Woods Cross and** West Bountiful City Master Plans have outlined plans for bike paths and equestrian trails in addition to sidewalks and have



Sidewalk & stamped concrete park strip near the UPRR crossing

identified the corridor as a bicycle tour route. Interviews with residents and businesses along the corridor have also indicated need for consistent sidewalks along the SR-68 corridor.

#### 1.7 Right-of-Way

Right-of-way along the corridor, from the intersection of 2600 South and Redwood Road to approximately the intersection of 800 West and 500 South, is 100 feet. From 800 West to I-15, the right-of-way width narrows to a total width of 66 feet.

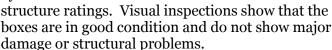
#### 2.0 Pavement

Pavement conditions within the study area appear to be fair to poor. UDOT performed a Visual Distress Survey on the pavement for the 500 South portion of the project in 2001. A visual inspection showed rutting near the UPRR crossing and extensive transverse and longitudinal cracks throughout most of the project area with a marginal skid resistance index. There are a few asphalt patches, which are most likely from utility work in the area. The pavement is near the end of its life cycle and is expected to fail within the next 10 years.

#### 3.0 Structures

There is a minor structure in the study area. This is the box culvert that carries Mill Creek. The box is a complex double-barrel box culvert (one is 10' x 4' and the other is 8' x 4') and is approximately 550

feet long. The culverts begin together on the south side of 500 South and west of the I-15 southbound ramps at the detention basin. The boxes split with the 8' x 4' box running along the south side of 500 South and angling north at 700 West. The 10' x 4' box runs under 500 South and turns west on the north side of 500 South, paralleling the 8' x 4' box. The boxes join together 200 feet north of 500 South at 700 West. These structures are not considered major





Box culverts carrying Mill Creek under 500 South

by UDOT and therefore do not have

#### 4.0 Drainage

The existing drainage system for the SR-68 corridor is inconsistent. Between I-15 and the D&RGW railroad, concrete curb and gutter carries

runoff to the storm drain catch basins. There is intermittent curb and gutter west of the D&RGW railroad. At the ends of the curb and gutter sections, runoff is allowed to drain to the roadside swales. Erosion has occurred at these locations and has undermined the curb sections. On the north side of 500 South, the storm drain system consists of open ditches, reinforced concrete pipes and corrugated metal pipes. These pipes range in size from 18 to 48 inches. See figures D1 to D12 for pipe

locations and sizes. At 1100 West, a 48-inch culvert drains into an open ditch. This open ditch discharges into the A1-A canal. The south side of 500 South has a mix of open ditches and 18-inch concrete pipes. This system also drains into the A1-A canal.

The 48-inch pipe is adequate for the drainage area,

however, some portions of the original drainage system installed in the 1950's are still being utilized. As development has taken place along the corridor, some improvements have been made to the system, but the entire system has never been

> replaced. The existing metal pipe along the corridor will most likely not meet the 50-year design life.

Along Redwood Road, there are no storm drain pipes except for a few cross drain pipes. See Figure D1 to Figure D12 for these locations. The existing storm drain system includes roadside swales and open ditches. The extremely flat grade along the roadway actually allows the roadside ditches to act as linear detention ponds as the storm water collects and slowly drains to the

north. This ponding can be observed just north of the airport. This water allows for vegetation to grow and encroaches on the clear zone area.

Sheet flow runoff from the Utah Auto Auction parking lot found on the north side of 500 South. just east of the curve, runs into the roadside ditches fronting the property. Also, an existing

> pond, just east of the parking lot, is fed by an artesian well, but collects no storm water.

A manhole, just west of the DR&GW rail crossing, overflows during high intensity storms. The manhole is full of mud and needs to be maintained. Many pipes in the area have been partially filled with silt. Maintenance crews have not found a blockage in the pipe to date.



Pond at 1650 West fed by an artesian well

#### 5.0 Irrigation/Canals Drainage along 500 South and

Redwood Road is presently at a poor level. Although Bountiful City has no master plan, the drainage system and its maintenance is a high priority. The A1-A drain is a major drainage feature that crosses 500 south with a corrugated metal pipe culvert that stretches from right-of-way to right-of-way. From a letter dated February 3, 1999, the United States Department of the Interior made

the comment: "...the design capacity of the A1-A drain is currently exceeded by unauthorized urban use of the drain, which is limiting our ability to properly drain agricultural lands."

The Mill Creek box culvert was upgraded a few years ago and at that time the existing box was abandoned and the creek was diverted to the north in a concrete lined channel. The abandoned culvert is still being used for roadway runoff. This culvert is not maintained and is filling up with silt and debris. Davis County would like to abandon the existing debris basin near 500 south and I-15. There are future plans to eliminate the basin and tie the box culvert under I-15 to

the 500 South box culvert crossing.

#### 6.0 Utilities

There is a variety of utilities that run along the SR-68/500 South corridor, including natural gas, culinary water, sanitary sewer, overhead power, overhead telecommunications, fiber optics, storm drain and petroleum pipe lines that serve the refineries in the area. There are several providers along the corridor for gas, water,

irrigation and storm drainage. There are also two railroad crossings on the corridor. The utility lines run throughout the entire corridor. See Figures B1 to B12 for approximate utility locations.

Questar Gas has a 6-inch high pressure gas line on the south side of the project that runs the length of the project corridor. There is a 2- to 4-inch steel line that runs parallel to the 6-inch line from 1150 West to Redwood Road. There is a 4-inch steelwrapped line that runs in the center of 500 South from I-15 to 1100 west.

Chevron Gas has two gas lines along 500 South. There is an 8-inch gasoline line that runs north and south along 1100 West and crosses 500 South. North of 500 South, there is an 8-inch diesel line, that runs parallel to the 8-inch gasoline line but turns and runs under 500 South to the Auto Auction where it turns south.

Holly Refinery has five lines in 10- and 12-inch ducts that cross 500 South at 800 West. The pipes range from 6 to 8 inches.

Utah Power has a substation located at the southwest corner of 500 South and the UPRR tracks. The power substation is located in a depression and Utah Power has expressed an interest in raising the substation grade. Utah Power has overhead distribution and transmission lines that run along the south side of 500 South. There is also a power line that runs parallel to the Union Pacific Railroad and crosses 500 south at 800 West.



Overhead power lines along 500 South

Qwest has a main duct that is located along the south side of 500 South. On the north side, Level 3 Communications has three fiber optic ducts and three separate buried feeder cables of 600 and 900 pairs.

#### Water

Woods Cross and West Bountiful cities manage separate distribution loops and trunk lines for water distribution and fire protection along the 500 South corridor. There are

numerous fire hydrants and water valves located within the right-of-way. These existing fire hydrants are in excellent condition.

West Bountiful has an existing 6- to 8-inch water line eight feet north of the existing edge of pavement. The city has plans to abandon the existing water line and install a new 12-inch water line.

#### Sewer

The sewer lines in the project corridor are under the jurisdiction of the South Davis County Sewer Improvement District. There is an 8-inch sewer line located on the north side of 500 South that runs from 1100 West to Redwood Road, a 12-inch line from 1100 West to 800 West, and a 10-inch line from 800 West to I-15. There are several major lines that tie into 500 South. They include: 21-inch line at 100 West; 10-inch line at 1320 West, just west of the Westwood Mobile Home

Community; and an 18-to 24-inch line that runs along Redwood Road and connects into the 10-inch line at 500 South.

#### 7.0 Crash Data

The UDOT Traffic and Safety Division generated a 3-year crash history for the project corridor from 2001 to 2003. The crash history results are summarized below. The actual severity and crash rate have been calculated for this segment of SR-68 from reported/documented crashes over the three year study period. Typically, the actual rates are compared to the expected rate to provide a relative measure of how safe a road might be presently operating. In order to derive an expected

Actual & Expected Crash Rates for Utah's Urban Principal Arterials							
URBAN PRINCIPAL ARTERIAL		ACTUAL					
	'01	'02	<b>'</b> 03	TOTAL/ AVE.			
Number of Crashes	21	26	27	74/24.7			
Crash Rate	2.92	2.43	2.52	2.62	3.96		
Severity	1.48	1.77	1.52	1.59	1.70		

Table 3: Crash History & Actual Crash Rates v. Expected Rates

SR-68 CRASH RATES (2001 to 2003)						
TYPE OF CRASH	PERCENT OF TOTAL	TOTAL NUMBER				
Rear End Crash	33.8%	25				
Right Angle Crash	25.7%	19				
Single Vehicle Crash	14.9%	11				
Left Turn Crash	13.5%	10				

Table 4: Types of Crashes v. the Total Number of Crashes

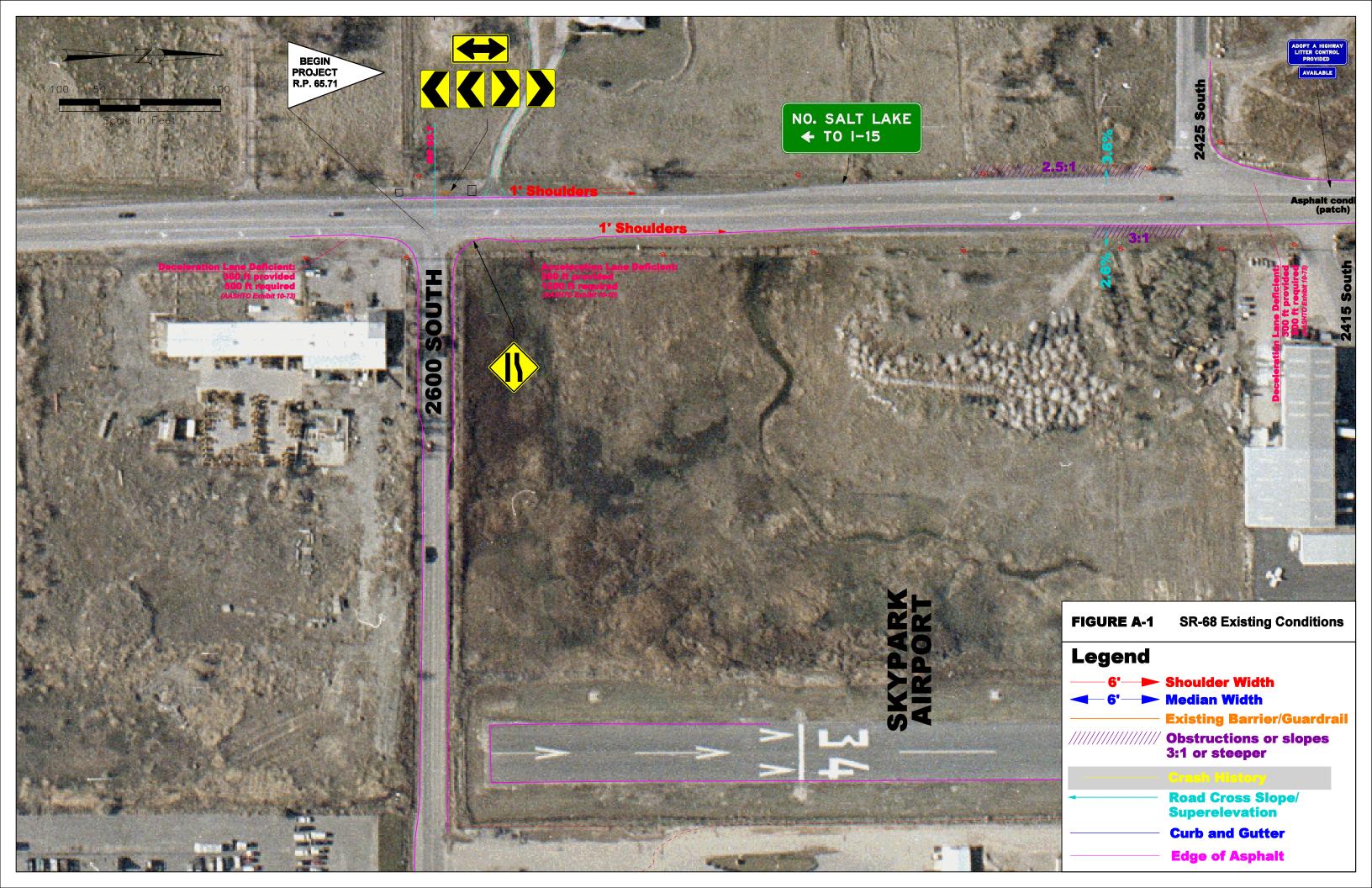
rate, UDOT calculates severity and crash rates for similar types of facilities within the state and averages them to produce these rates. On a facility similar to SR-68 across the entire state of Utah, the average expected severity rate was 1.70 and the average crash rate was 3.96. See Table 3 below for more detailed crash severity information.

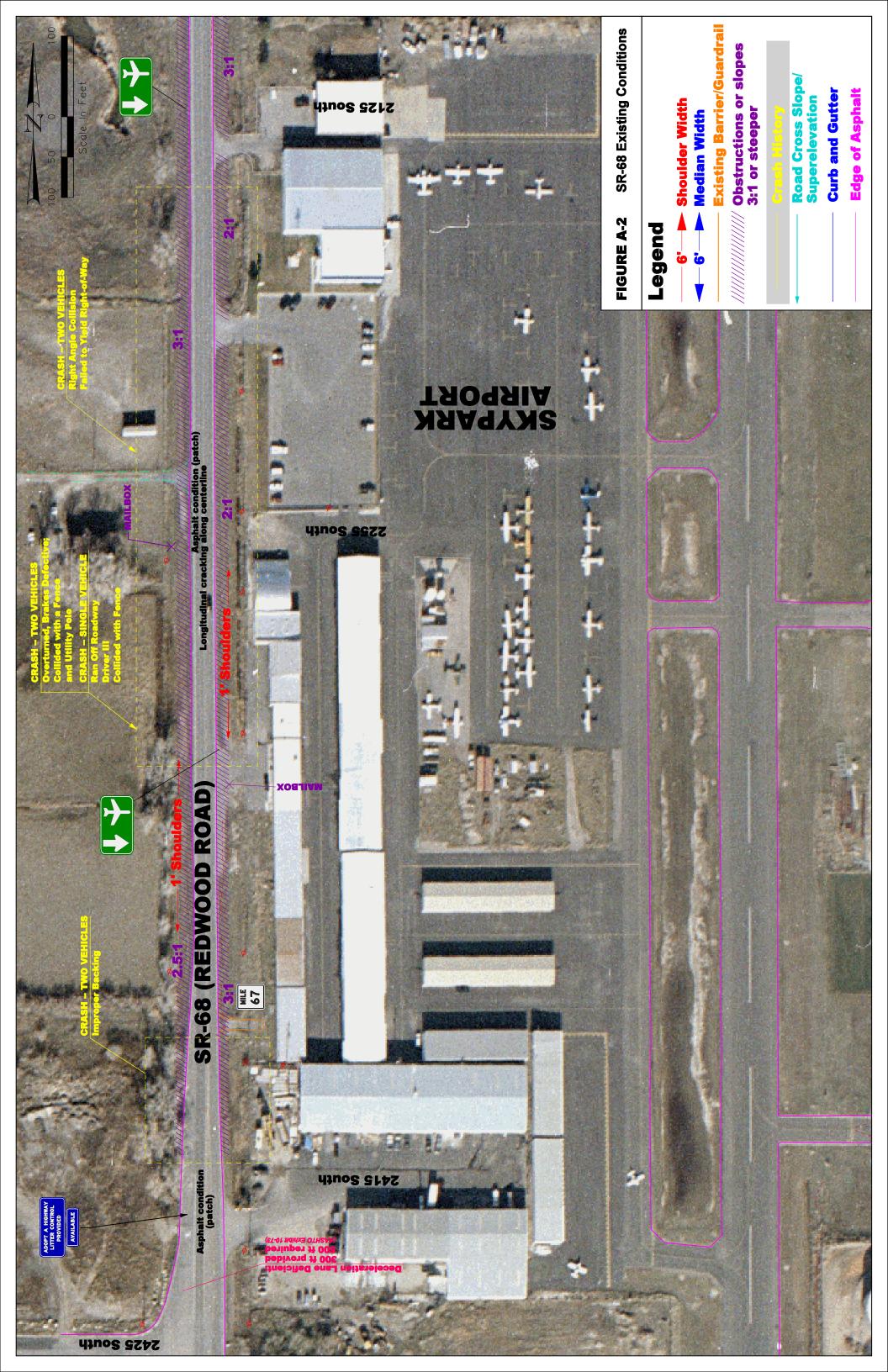
The expected crash rate is based on the state wide expected severity and number of accidents for a certain classification of roadway. SR-68 is classified as an urban arterial. As shown below, there is a lower occurrence of accidents and severity than would be expected for this classification of roadway. Refer to Tables 4 and 5 below and Figures A1 to A12 for detailed information of crash types. As shown in Tables 4 and 5, a high occurrence of rear-end and right angle collisions have occurred. These types of crashes suggest inadequate auxiliary lanes (right and left turn lanes, medians, and shoulders), or high travel speeds for roadway conditions. A review of these crashes and locations are shown in Figures A1-A12.

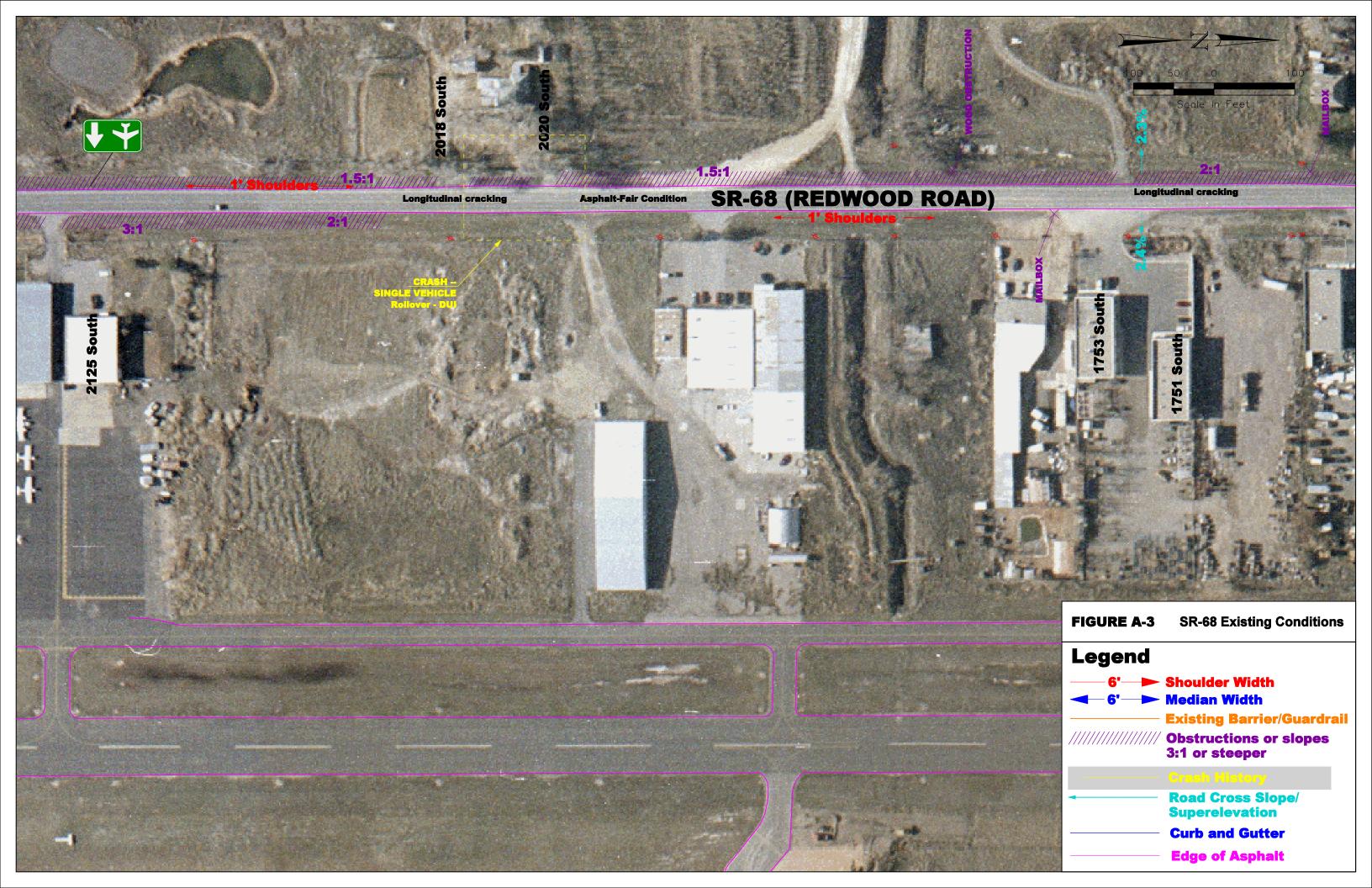
SR-68 CRASH LOCATIONS SUMMARY						
INTERSECTION						
CRASH TYPE	NUMBER					
1100 West & 500 South						
Right Angle Rear End	8 3					
800 West & 500 South						
Right Angle	4					
I-15 Southbound Ramps						
Left Turn Rear End Right Angle	9 6 4					

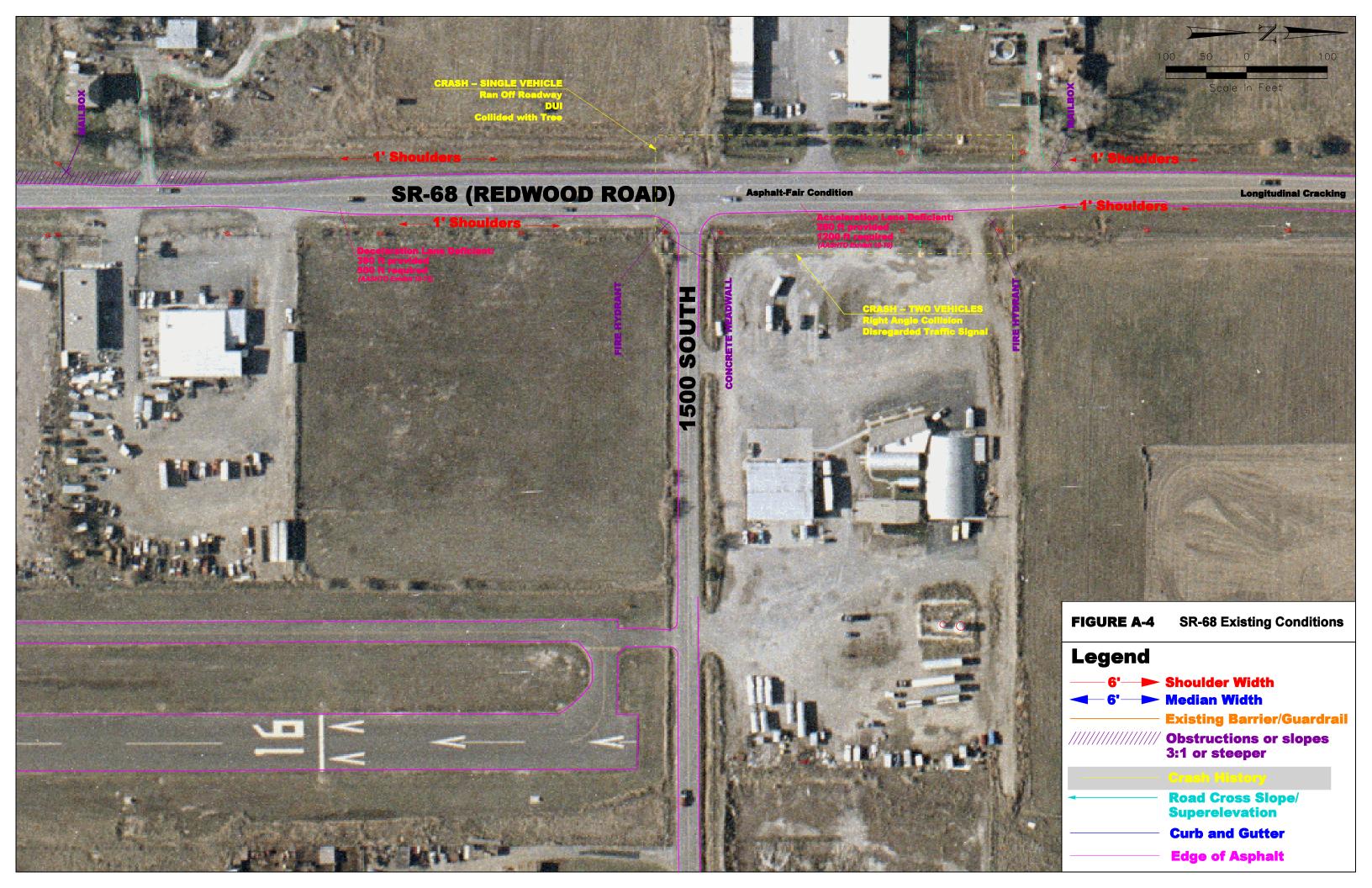
*Table 5: Locations with highest number of crashes* 

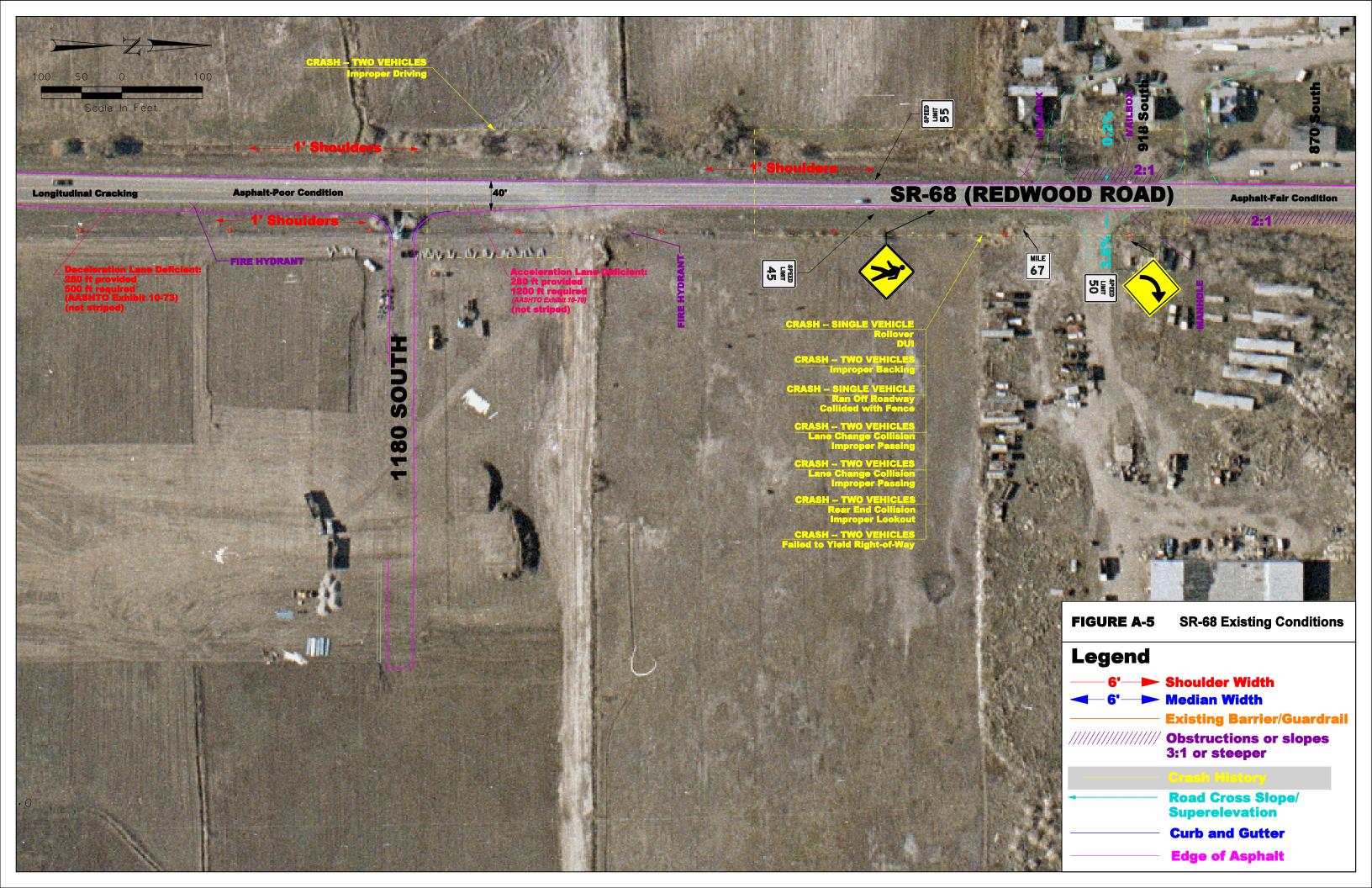
# **Appendix**

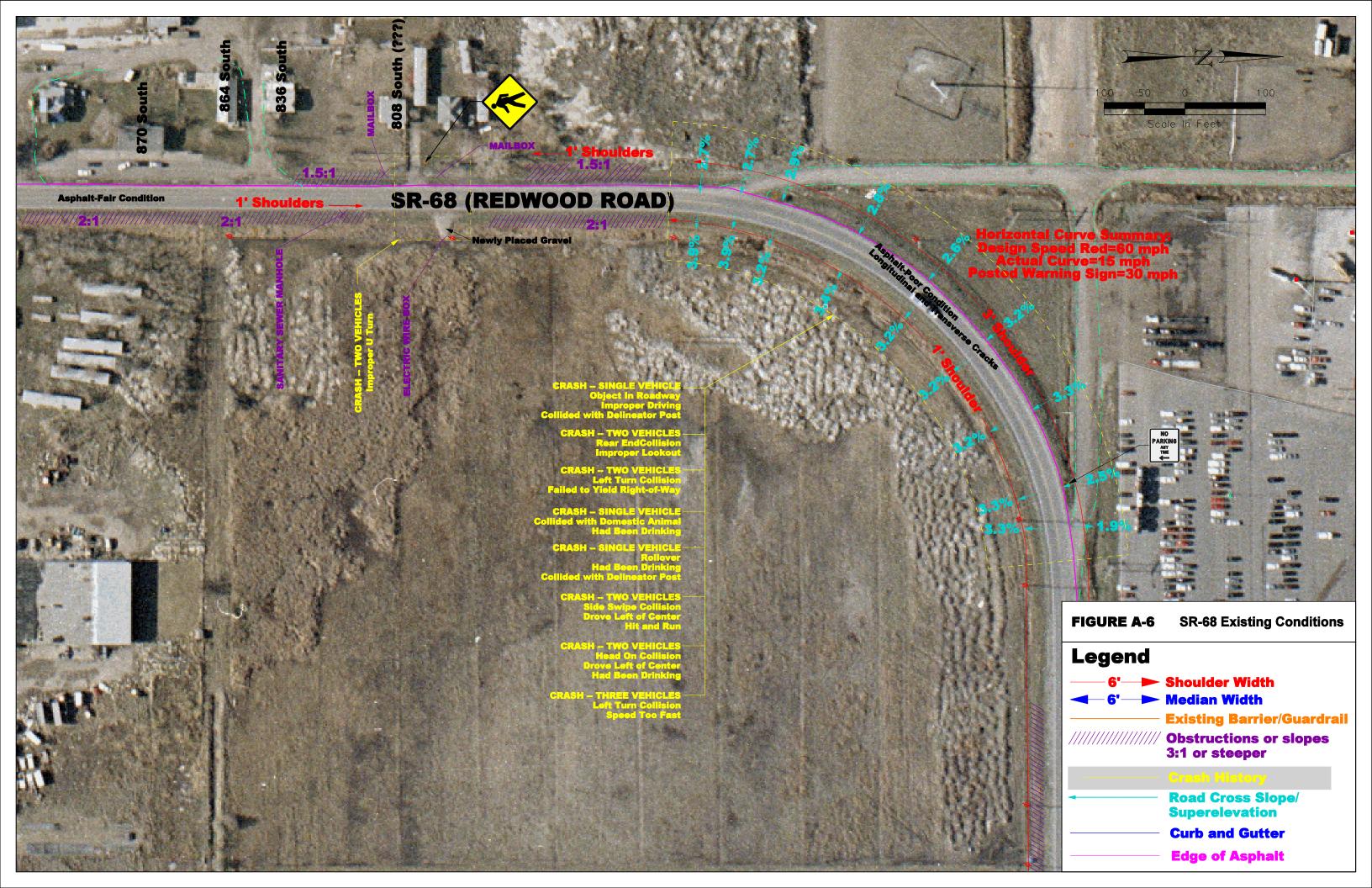


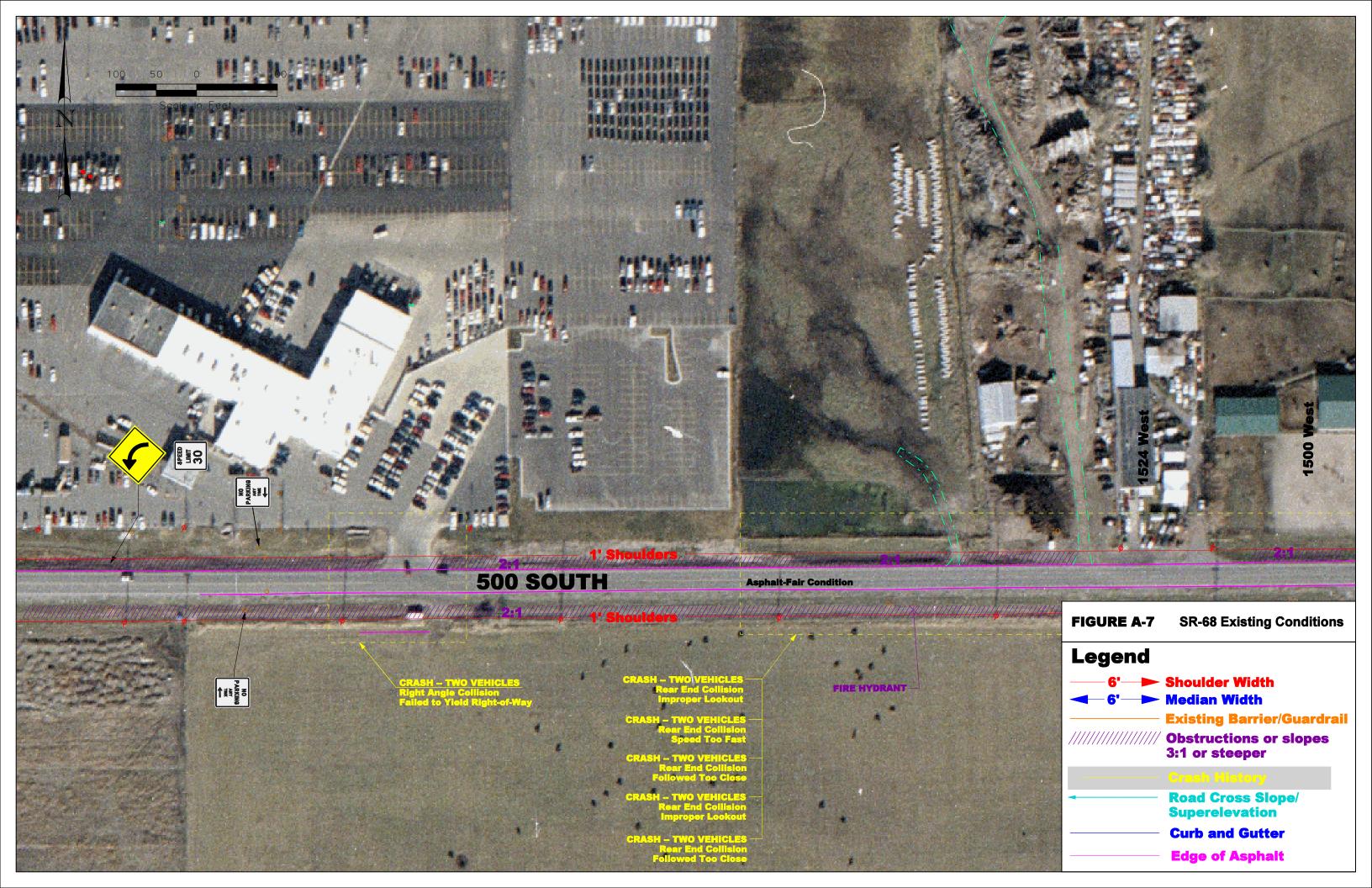


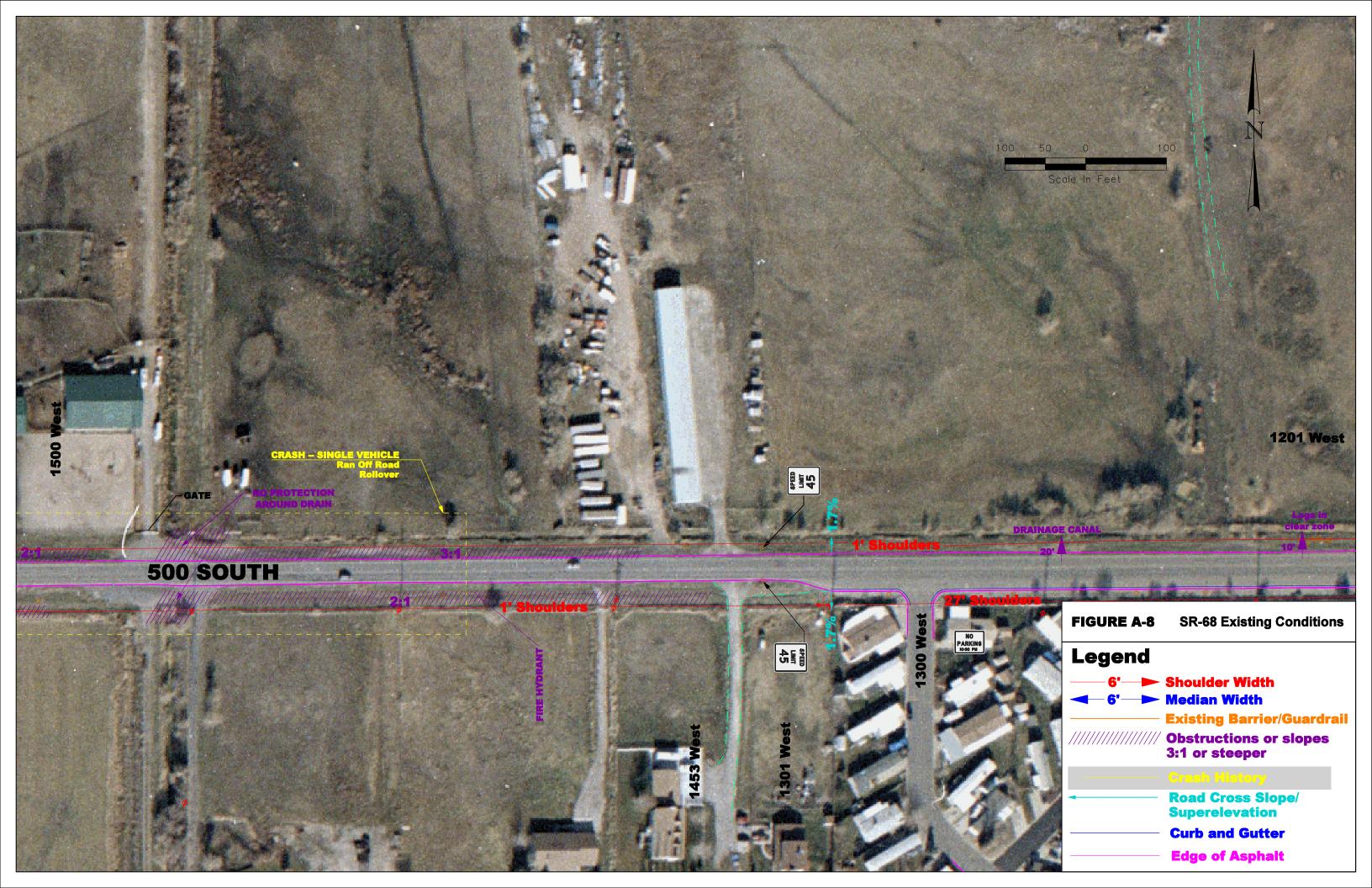


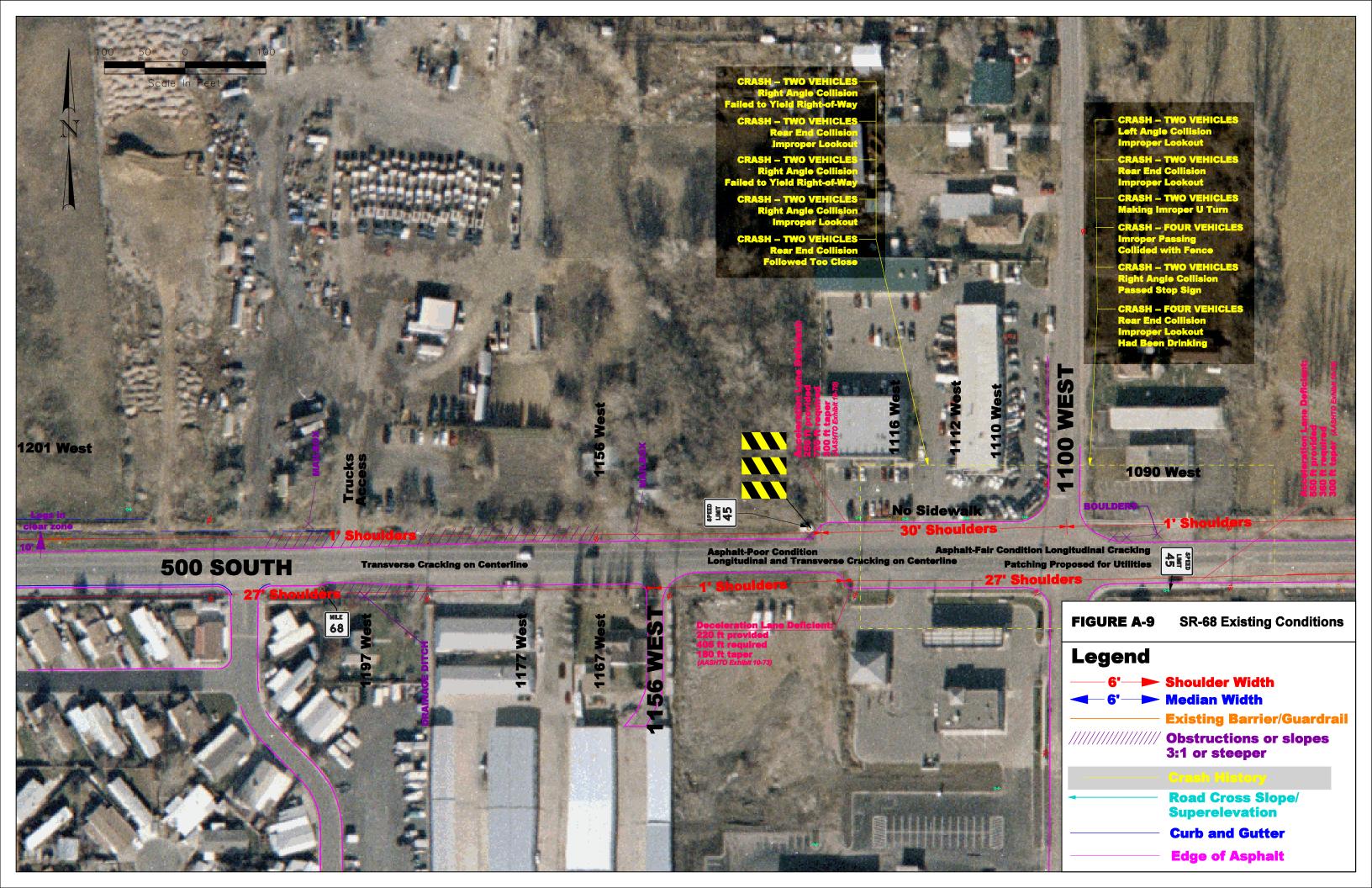


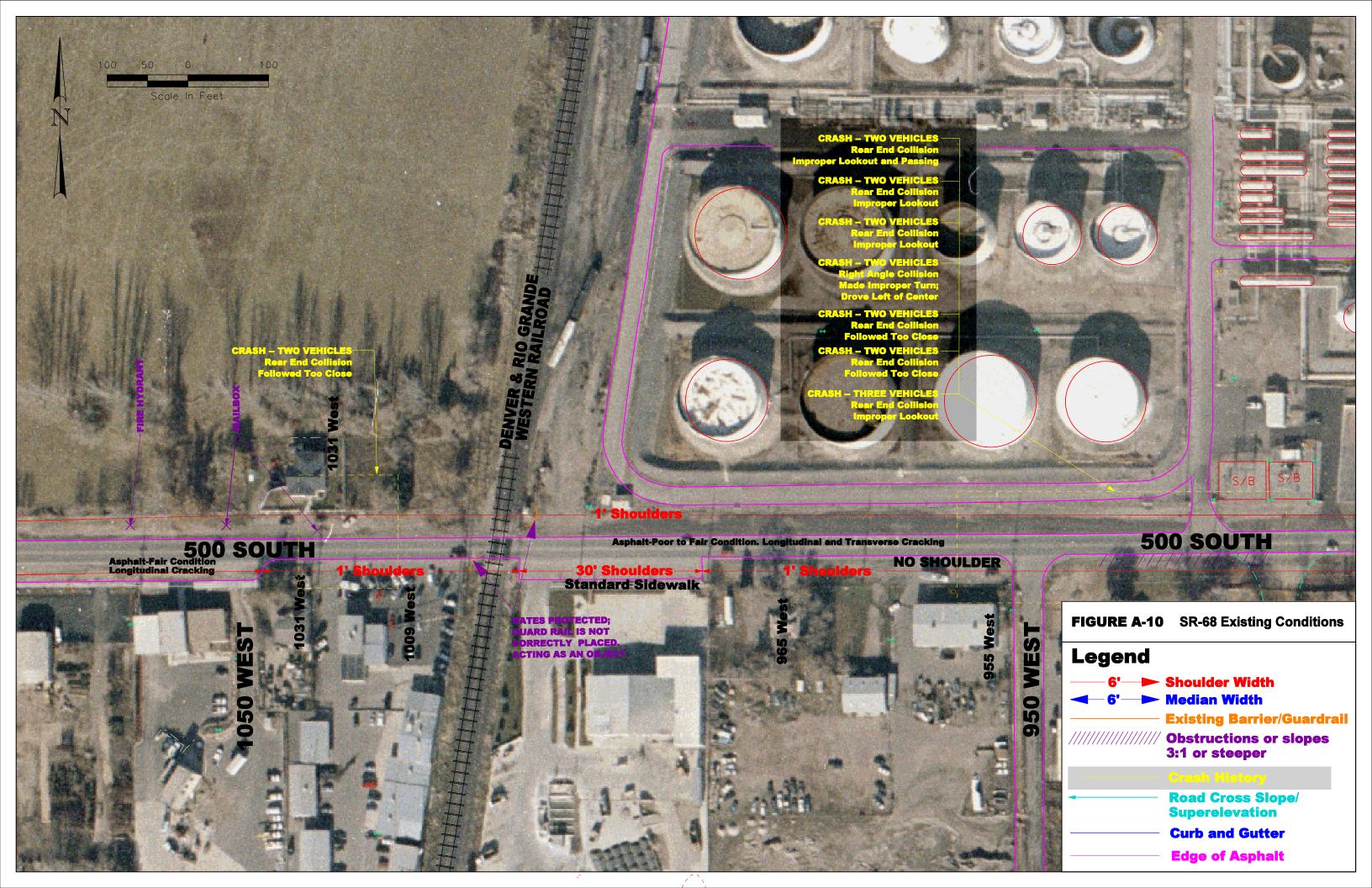


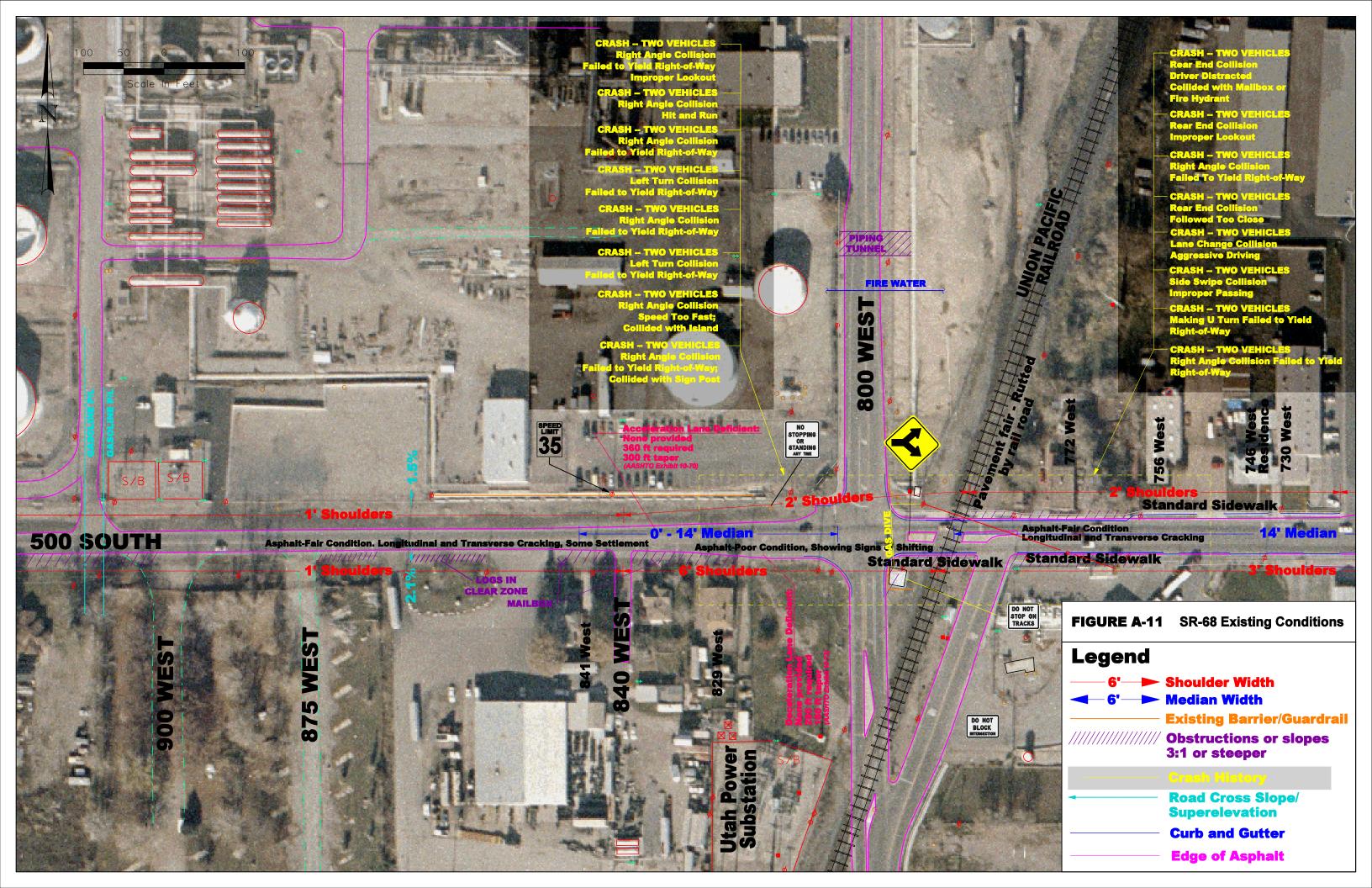


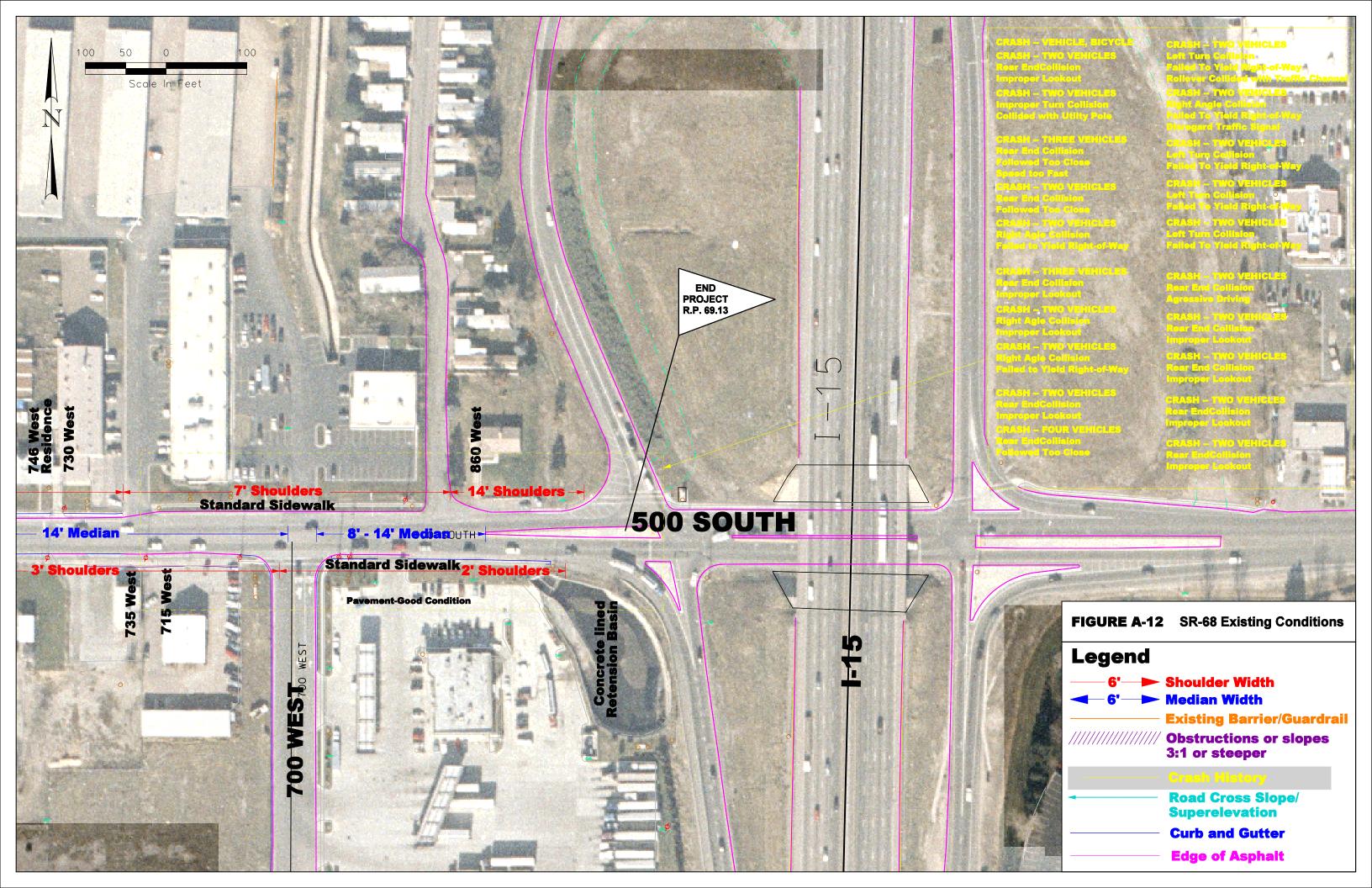


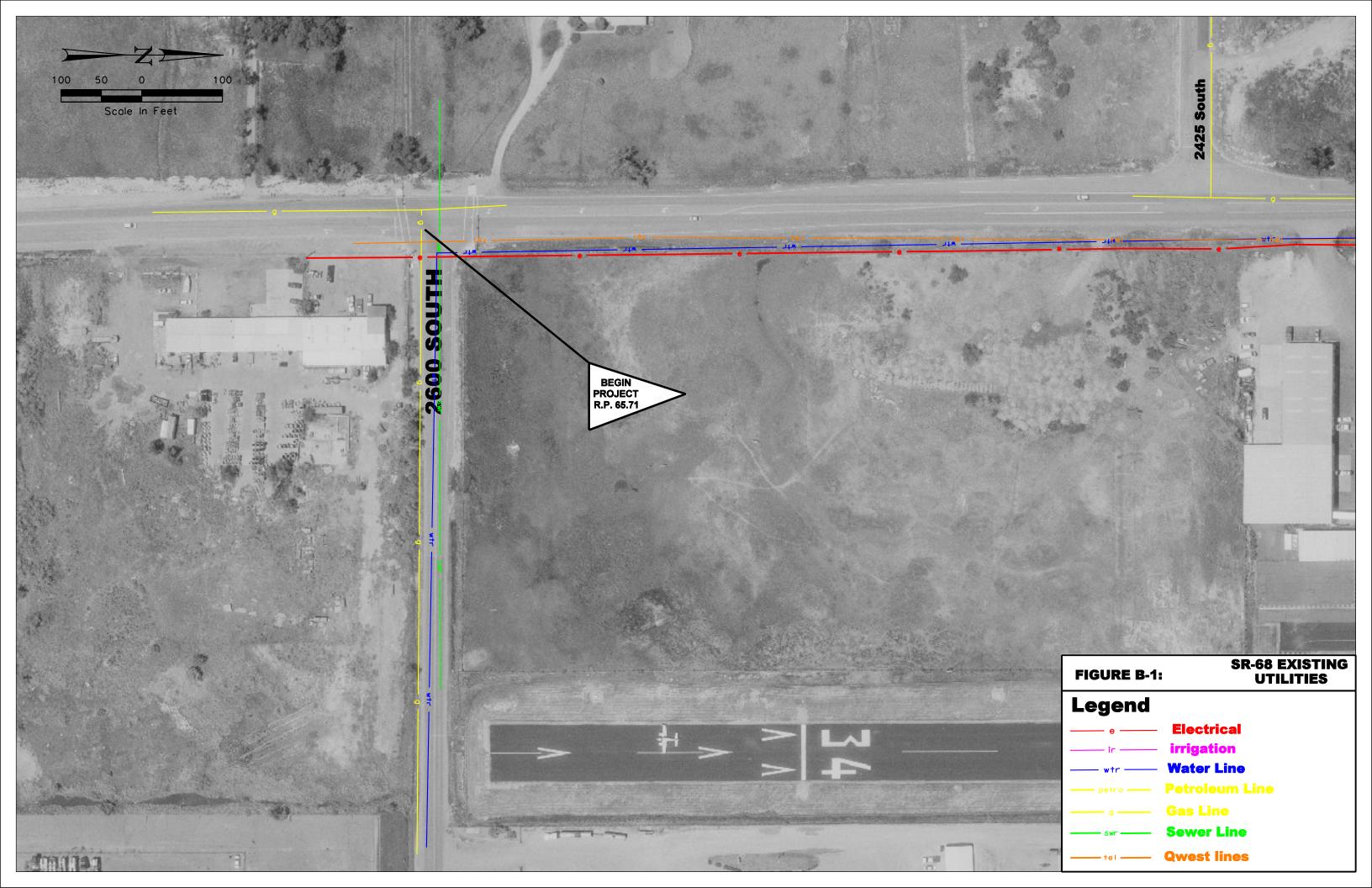






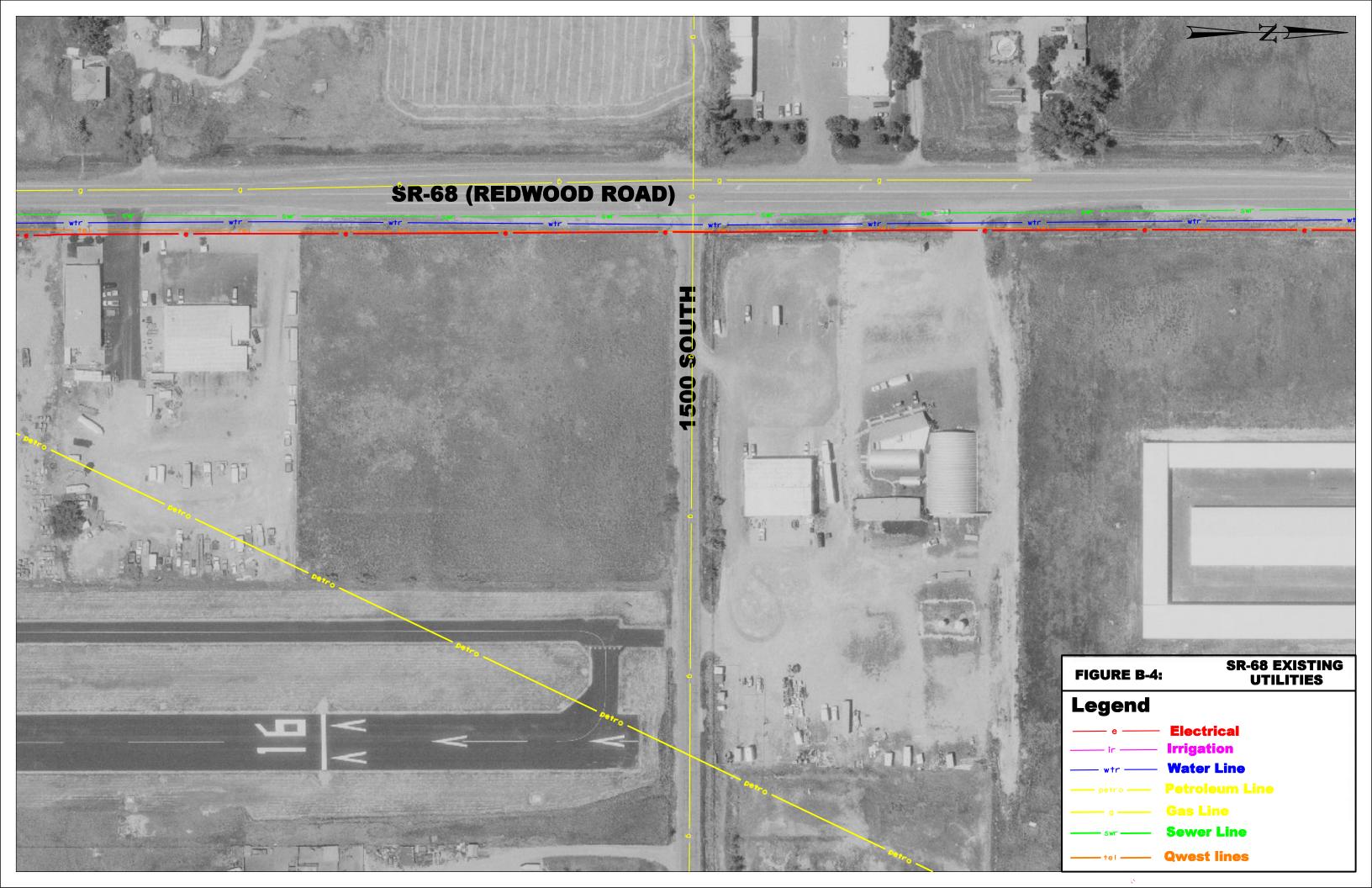




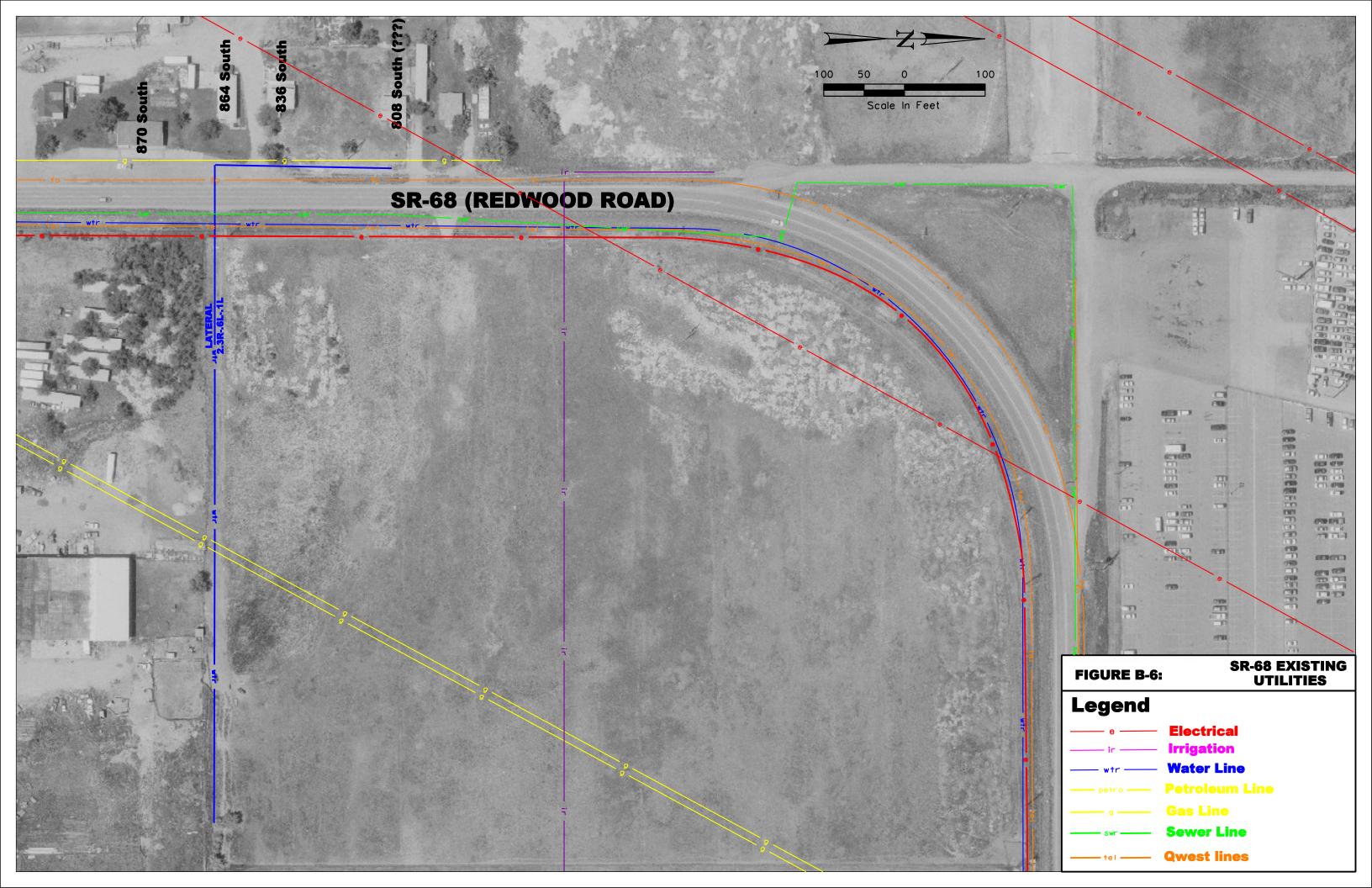








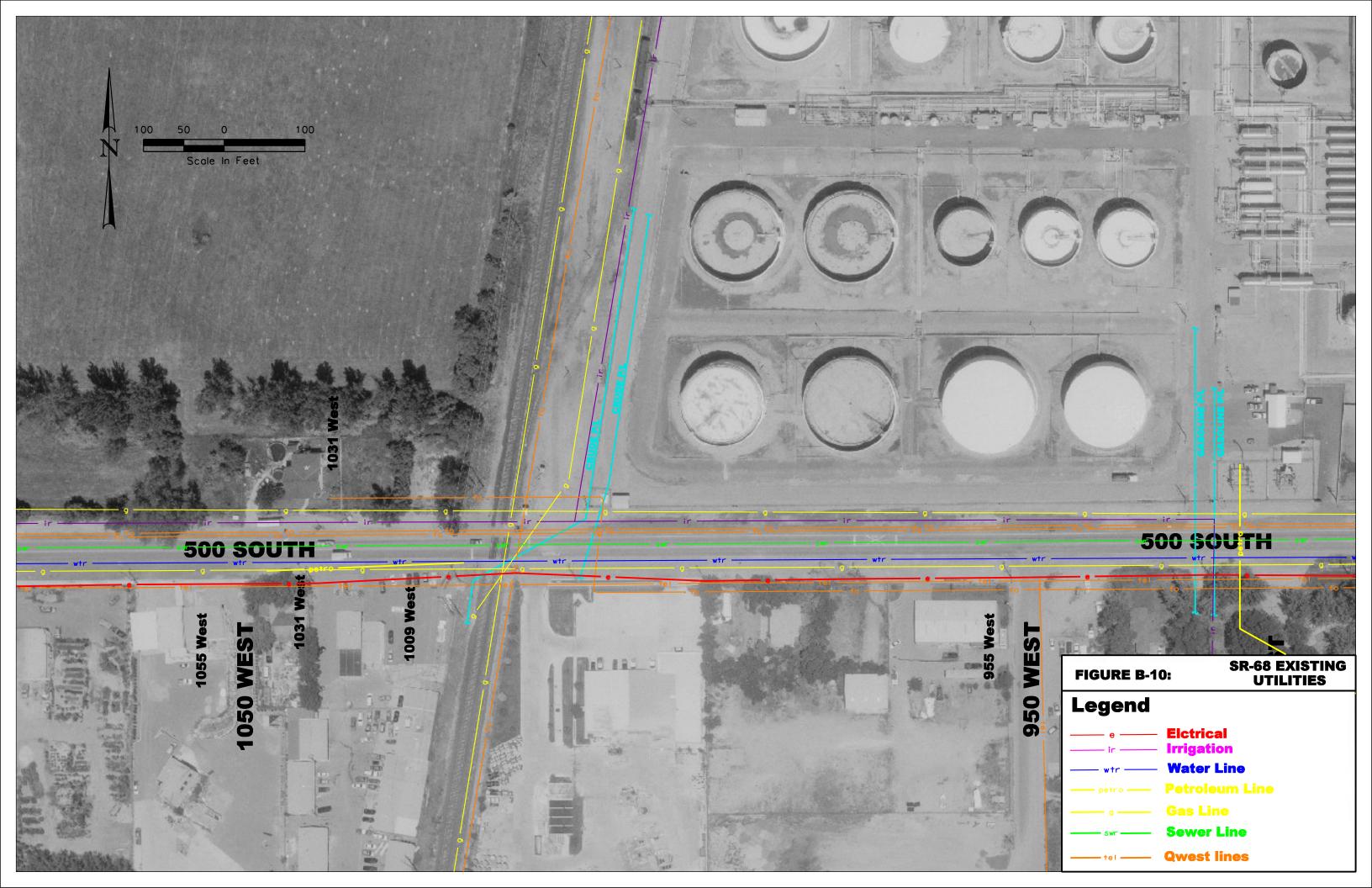


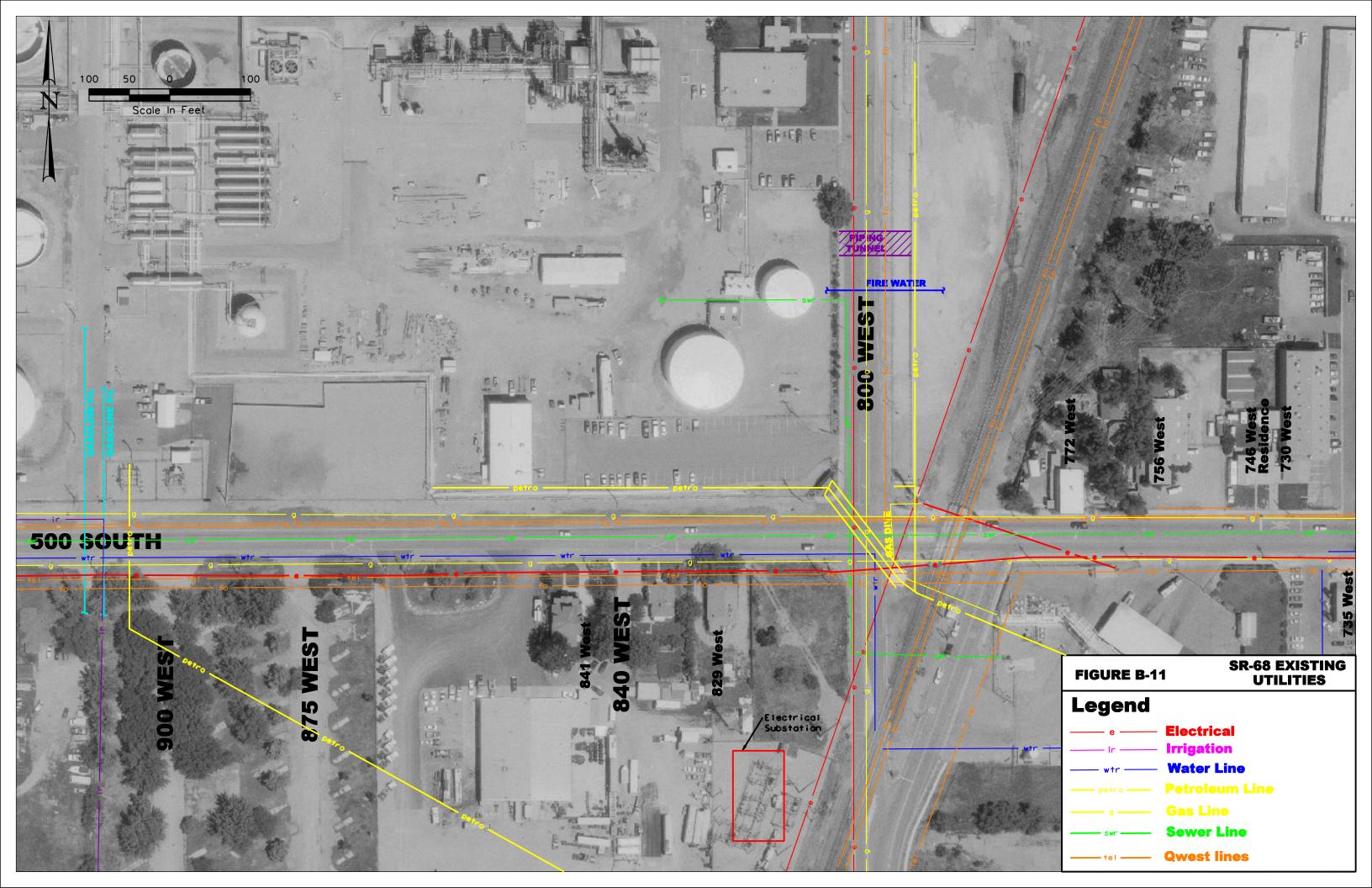


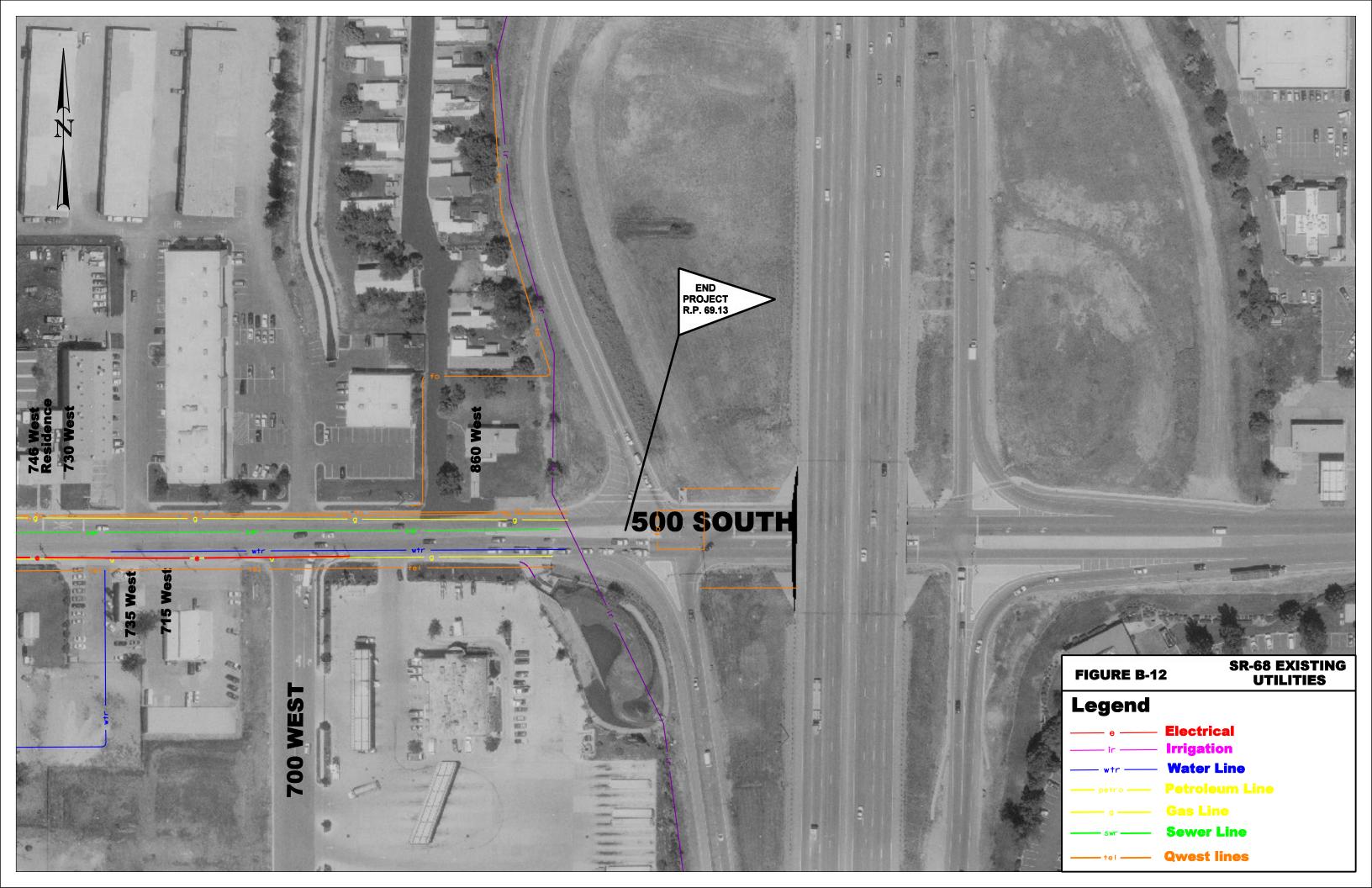












#### SR-68 -- Geometric Design Criteria

#### **DRAFT**

CRITICAL ELEMENT	AASHTO	UDOT	SR-68	Comments	AASHTO Ref.	UDOT Ref.
Functional Classification						
	Urban Principal Arterial	Urban Principal Arterial	Urban Principal Arterial		p. 11	Std. Dwg. GW 11: Note 1
1.00						
LOS Urban Arterial	С	C	С	Min. LOS=D	p. 84, 85; Exhibit 2-32 & p. 470	Ctd Dura DD 11; Note 1
Orban Arterial	C	C	<u> </u>	Min. LUS=D	p. 64, 65, Exhibit 2-32 & p. 470	Std. Dwg. DD 11: Note 1
Design Vehicle	1	1			1	
Mainline	BUS-40 - WB-109D	WB-62	WB-62		p. 18 & p. 42	Utah Reg. for Legal and Perm. Veh.
Minor Intersections	WB-40 - WB-50	WB-40 - WB-50	WB-40		p. 43	Utah Reg. for Legal and Perm. Veh.
Heavy Industrial Driveways	BUS-40 - WB-109D	BUS-40 - WB-62	WB-67		p. 18 & p. 42	Utah Reg. for Legal and Perm. Veh.
Design Speed (mph)						
R.P. 65.7-67.0	30 - 60	30 - 60	60	Posted speed is 55 mph	p. 470	Std. Dwg. DD 11: Note 1
R.P. 67.0 - 1100 West	30 - 60	30 - 60	50	Posted speed is 45 mph	p. 470	Std. Dwg. DD 11: Note 1
1100 West - I-15	30 - 60	30 - 60	40	Posted speed is 35 mph	p. 470	Std. Dwg. DD 11: Note 1
Lane Widths (ft)						
Thru Lane	12	12	12		p. 472 & p. 473	Std. Dwg. DD 11: Note 1
Turn Lane/Median	4 - 18	14	14		p. 474	Std. Dwg. ST 5
Shoulder Widths (ft)					1	
2-Lane Hwy	4	4	4		Exhibit 7-3, p. 448	Std. Dwg. DD 11: Note 1
Multi-Lane Hwy	8	8	8		p. 455	Std. Dwg. GW 11
Minimum shoulder	2 + 2	2 + 2	2 + 2	2' Paved, 2' Base	p. 448	Std. Dwg. DD 12: Note 1
Horizontal Alignment		·				
R <sub>min</sub>	Exhibit 3 - 26	AASHTO Exhibit 3 - 26	Exhibit 3 - 26	Assume 6% max	p.168	Std. Dwg. DD 11: Note 1
HSO (Horizontal Sight Offset)	Exhibit 3 - 53	AASHTO Exhibit 3 - 53	Exhibit 3 - 53		p. 226	Std. Dwg. DD 11: Note 1
Vertical Alignment						
Crest	Exhibit 3 - 71	AASHTO Exhibit 3 - 71	AASHTO Exhibit 3 - 71		p. 271	Std. Dwg. DD 11: Note 1
Sag	Exhibit 3 - 74	AASHTO Exhibit 3 - 74	AASHTO Exhibit 3 - 74		p. 275	Std. Dwg. DD 11: Note 1
Grades						
Minimum	0.5%	0.3%	0.5%		p. 242	Std. Dwg. DD 12: Note 9
Maximum	Exhibit 7-10	AASHTO Exhibit 7-10	AASHTO Exhibit 7-10		p. 472	Std. Dwg. DD 11: Note 1
Sight Distance						
Stopping Sight Distance	Exhibit 7-1	AASHTO Exhibit 7-1	AASHTO Exhibit 7-1		p. 445	Std. Dwg. DD 11: Note 1
Passing Sight Distance Intersection Sight Distance	Exhibit 7-1 Ex. 9-51, 55, 58, 61, 64, 67, 70	AASHTO Exhibit 7-1 Ex. 9-51, 55, 58, 61, 64, 67, 70	AASHTO Exhibit 7-1 Ex. 9-51, 55, 58, 61, 64, 67, 70		p. 445, p. 449 p. 650-682	Std. Dwg. DD 11: Note 1 Std. Dwg. DD 11: Note 1
Cross Section						
Cross Slope Clear Zone	1.5% - 2% Table 3.1	2% Roadside Design Guide Table 3.1	2% Roadside Design Guide Table 3.1		p. 143, 450 & 459 RDG p 3-6 Table 3.1	Std. Dwg. DD 11 Std. Dwg. DD 11: Note 2
Sideslopes	2:1 - 10:1	2:1 - 6:1	2:1 - 6:1		RDG p 3-6 Table 3.1	Std. Dwg. DD 11. Note 2
·					·	
Superelevation  Maximum Rate	Exhibit 3 - 26	AASHTO Exhibit 3 - 26	Exhibit 3 - 26	Assume 6% max	p.168	Std. Dwg. DD 11: Note 1
Runoff Rate	Exhibit 3 - 32	AASHTO Exhibit 3 - 32	Exhibit 3 - 26	Assume 6% max	p.178 & 181	Std. Dwg. DD 11: Note 1
Smood Change Lance						
Speed Change Lanes Accel. Lanes	Exhibit 10-70	AASHTO Exhibit 10-70	AASHTO Exhibit 10-70		p.688-689, 847	Std. Dwg. DD 11: Note 1
Decel. Lanes	Exhibit 10-73	AASHTO Exhibit 10-73	AASHTO Exhibit 10-73		p.688-689, 851	Std. Dwg. DD 11: Note 1
Structural Capacity						
C. Lotarai Capacity	HS - 20	HS - 20	HS - 20		p. 447	Std. Dwg. DD 11: Note 1
Vertical Observation						
Vertical Clearance Roadway	16'-0"	17'-0"	17'-0"		p. 472	Std Dwg DD8
Railroad	-	23'-6"	23'-6"		p. 472	Std. Dwg. DD 10
Delder Wilde						
Bridge Width	Roadway Width	Roadway Width + 4'	Roadway Width + 4'		p. 481	Std. Dwg. DD 9
AASHTO Ref = Geometric Design of Hig		Hoddwdy Width T T	Housing Wilder T		p. 401	5.td. 5.mg. 55 9
RDG = AASHTO Roadside Design Guid	e, 2002 Edition					

# Horizontal Curve Sight Distance Check **DRAFT**

#### Bold = Design Standard Criteria Not Achieved

Alignment	Curvo No	Direction	Approx.	PI Station	Design Speed	Radius	Min. Radius	Superelevation		Sight Distance Required Provided		Radius - Lane at Inside of	Horizontal Sightline Offset		Restrictive Sight	Comments	
Aligninent	Ourve No.		Mile Post	1 1 Otation	Opecu		Tiadius	Required	Provided	riequireu	riovided	Curve	Provided	Required	Distance Feature	Confinents	
					MPH	FT	FT	%	%	FT	FT	FT	FT	FT			
SR-68	1	Left	65.77	15+69.677	60	12000	12000	-2.0%	-2.0%	570	570 +	11980	30.00 +	0.243	None	Existing alignment	
SR-68	2	Right	65.98	27+14.696	60	12000	12000	-2.0%	-2.0%	570	570 +	11994	44.00 +	0.066	None	Existing alignment	
SR-68	3	Right	66.14	35+69.860	60	12000	12000	-2.0%	-2.0%	570	570 +	11994	44.00 +	0.007	None	Existing alignment	
SR-68	4	Left	66.50	54+22.194	60	12000	12000	-2.0%	-2.0%	570	570 +	11994	44.00 +	0.001	None	Existing alignment	
SR-68	5	Right	66.71	65+49.269	60	12000	12000	-2.0%	-2.0%	570	570 +	11994	44.00 +	0.025	None	Existing alignment	
SR-68	6	Right	66.82	71+17.231	50	8000	8000	-2.0%	-2.0%	425	425 +	7994	44.00 +	0.045	None	Existing alignment	
SR-68	7	Left	67.03	82+22.878	50	8000	8000	-2.0%	-2.0%	425	425 +	7994	44.00 +	0.012	None	Existing alignment	
SR-68	8	Right	67.26	94+52.369	50	500	3480	3.0%	3.0%	425	425 +	494	45.011 +	44.487	None	Existing alignment; emax = 6%	
SR-68	9	Left	67.47	105+44.575	50	8000	8000	-2.0%	-2.0%	425	425 +	7994	44.00 +	0.025	None	Existing alignment	
SR-68	10	Right	67.81	123+57.113	50	8000	8000	-2.0%	-2.0%	425	425 +	7994	44.00 +	0.004	None	Existing alignment	
SR-68	11	Left	67.99	133+01.075	50	8000	8000	-2.0%	-2.0%	425	425 +	7994	35.01	0.018	None	Existing alignment	
SR-68	12	Right	68.24	146+32.622	40	6000	6000	-2.0%	-2.0%	305	305 +	5994	44.00 +	0.019	None	Existing alignment	
SR-68	13	Left	68.47	158+75.397	40	6000	6000	-2.0%	-2.0%	305	305 +	5994	44.00 +	0.010	None	Existing alignment	
SR-68	14	Right	68.78	174+86.101	40	6000	6000	-2.0%	-2.0%	305	305 +	5980	30.00 +	0.157	None	Existing alignment	

# Vertical Curve Criterial Check **DRAFT**

## **Bold = Design Standard Criteria not Achieved**

			PVI Station	Design		Gra	ıde		Vert.	K	-Value			
Alignment	Vertical	Approx. Mile Post			Max.	Min.	In	Out	Curve Type	Min.	Prov'd.	Length	Comments	
Aligninient	Curve No.				Req'd.	Req'd.	Prov'd.	Prov'd.		Req'd.	1 10v u.	Lengin		
					%	%	%	%						
SR-68	1	66.37	47+80.00	60	6.00%	0.30%	0.03%	-0.38%	Crest	151	730	300	Existing alignment	
SR-68	2	66.55	57+00.00	60	6.00%	0.30%	-0.38%	-0.18%	Sag	136	1242	250	Existing alignment	
SR-68	3	67.06	84+00.00	50	6.00%	0.30%	0.18%	0.11%	Sag	96	516	150	Existing alignment	
SR-68	4	67.49	107+00.00	50	6.00%	0.30%	0.11%	0.48%	Sag	96	2181	800	Existing alignment	
SR-68	5	67.92	129+60.00	50	6.00%	0.30%	0.48%	0.34%	Crest	84	7199		Existing alignment	
SR-68	6	68.21	144+60.00	40	6.00%	0.30%	0.34%	2.91%	Sag	64	195	500	Existing alignment	
SR-68	7	68.32	150+80.00	40	6.00%	0.30%	2.91%	0.74%	Crest	44	92		Existing alignment	
SR-68	8	68.43	156+20.00	40	6.00%	0.30%	0.74%	1.41%	Sag	64	1129		Existing alignment	
SR-68	9	68.62	166+40.00	40	6.00%	0.30%	1.41%	2.76%	Sag	64	370	500	Existing alignment	
SR-68	10	68.70	170+40.00	40	6.00%	0.30%	2.76%	1.43%	Crest	44	150	200	Existing alignment	

## Cross Section Element Check

### DRAFT

#### Bold = Design Standard Criteria not Achieved

Alignment	Beginning Station	Ending	Design	Lane Width		Clear Zone				Southb	ound/Westl	oound		Northbound/Eastbound						
			Speed					No. of	Side	Shoulder Shoulder	Parrier Pag'd	Barrier Prov'd	No. of	Side Slope	Shoulder	Shoulder	Barrier	Barrier		
		Station	Орсса	Req'd	Prov'd	Req'd	Prov'd	Lanes	Slope	Req'd	Prov'd	Darrier Mequ	Daillei Flov u	Lanes	Side Slope	Req'd	Prov'd	Req'd	Prov'd	
			mph	ft	ft	ft	ft	Laries	ft:ft	ft	ft				ft:ft	ft	ft			
SR-68	12+77	80+88	60	12	12	30	30*	1	6:1*	8	1	*	None	1	6:1*	8	1	*	None	
SR-68	80+88	142+77	50	12	12	20	20*	1	6:1*	8	1-30	*	None	1	6:1*	8	1-24	*	None	
SR-68	142+77	182+00	40	12	12	16	16**	1	6:1	8	1-14	*	None	1	6:1	8	1-24	*	None	

<sup>\*</sup> Typical clear zone provided is sufficient; however, there are various hazards inside the clear zone that need barrier or end treatments.

\*\* For urban arterials, collectors, and local streets where space is restricted, a min. distance of 18 inches should be provided from the curb face to object.

